



University Of Technology
Building and Construction Eng. Dept.
Final Exam 1st Attempt – 2015/2016

Subject : Land Surveying
Branch : Geomatic Engineering
Examiner : Tariq Naji

Class: 3^{ed}
Time : 3 hours.
Date : 13/6/2016



Answer Four Questions

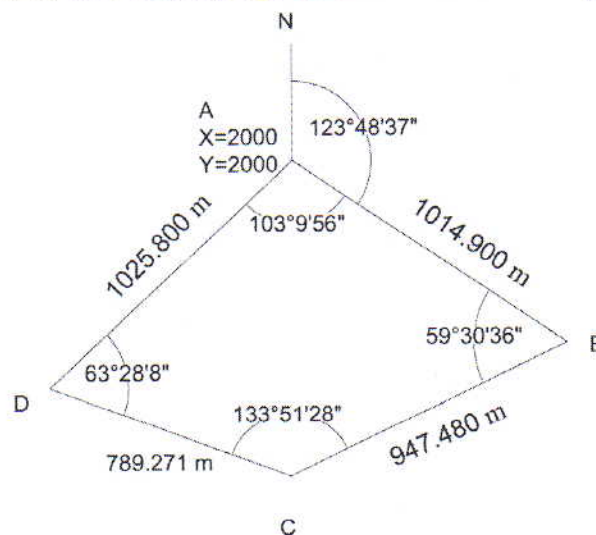
Q1) The following table represent the field data of profile leveling along a center line of irrigation channel.

Station	B.S	I.F.S	Distances			Ground Elevation	Grade Elevation
			L	CL	R		
B.M ₁	0.536					77.200	
		1.265		0+00			73.971
		2.124	2.50				
		1.492	5.00				
		1.994			2.50		
		2.179			5.00		
		1.488		1+00			
		2.882	2.50				
		1.691	5.00				
		1.594			2.50		
		2.421			5.00		

If the grade slope is +0.01 % , channel width =5 m and side slope = 1:1.5,

1. Draw the cross sections with scale 1/100 **15 marks**
2. Compute the area of cross sections **7 marks**
3. Compute the depth of cut and/or fill for all stations **3 marks**

Q2) For the following figure balance the departures and latitudes using the compass rule and computed the balanced coordinates of all points.



25 marks

Q3) Answer two of the following: 12.5 marks each

A) In triangulation systems what are the types of layouts, explain with figures.

B) Reciprocal leveling was conducted across a wide valley to determine the difference in level of points A and B, A situated on one side of the valley and B situated on the other. The following results on the staff held vertically at A and B from level stations 1 and 2, respectively, were obtained. The level station 1 was near to A and station 2 was near to B. If the elevation of B is 400.487 m above the MSL, what is the elevation of point A?

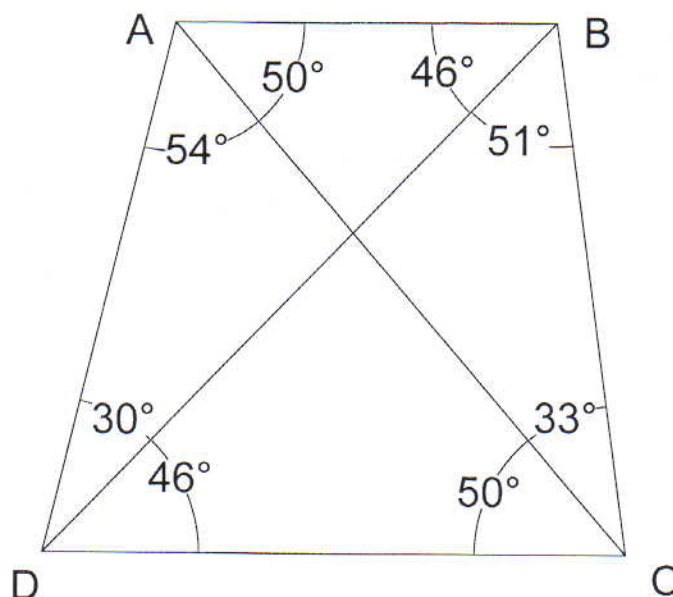
Instrument at	Staff reading on	
	A	B
1	1.641	2.193
2	2.141	2.701

C) A steel tape standardized at 20°C, the tape had a weight of 0.03967 kg/m. This tape was held horizontally, supported at the ends only, with a constant tension of 9.09 kg, to measure a line from A to B, $D_{AB}=50.205$. Apply corrections for tape sag, to determine the correct length of the line.

Q4) For the following figure ABCD:

A) Compute the strength of the figure ABCD for all the routes by which the length CD can be computed from the known side AB. Assume that all the stations were occupied. 15 marks

B) If the horizontal coordinates of point A=($X_A=10000$ m, $Y_A=10000$ m), length of line AB = $D_{AB}=5000$ m and Az of AB= $Az_{AB}= 190^\circ$. Compute the horizontal coordinates of the triangulation network using Δ^s **having the lowest value of R.**



10 marks

Q5) A) List the typical total station programs 9 marks

B) Using a programmed total station: 16 marks

- 1- How would the surveyor can determine the heights of inaccessible points.
- 2- How would you find the center location of a concrete column.



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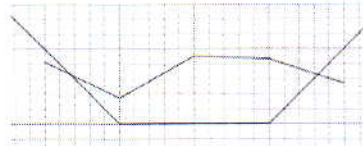
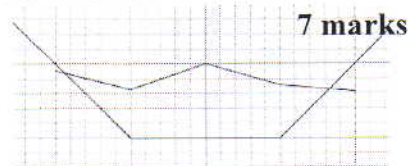
Answer Four Questions

Q1) The following table represent the field data of profile leveling along a center line of irrigation channel.

Station	B.S	I.F.S	Distances			Ground Elevation	Grade Elevation	Depth
			L	CL	R			
B.M ₁	0.536					77.200		
		1.265		0+00		76.471	73.971	2.5
		2.124	2.50			75.612		1.641
		1.492	5.00			76.244		2.273
		1.994			2.50	75.742		1.771
		2.179			5.00	75.557		1.586
		1.488		1+00		76.248	73.981	2.267
		2.882	2.50			74.854		0.873
		1.691	5.00			76.045		2.064
		1.594			2.50	76.142		2.161
		2.421			5.00	75.315		1.334

If the grade slope is +0.01 % , channel width =5 m and side slope = 1:1,

1. Draw the cross sections with scale 1/100 15 marks



2. Compute the area of cross sections

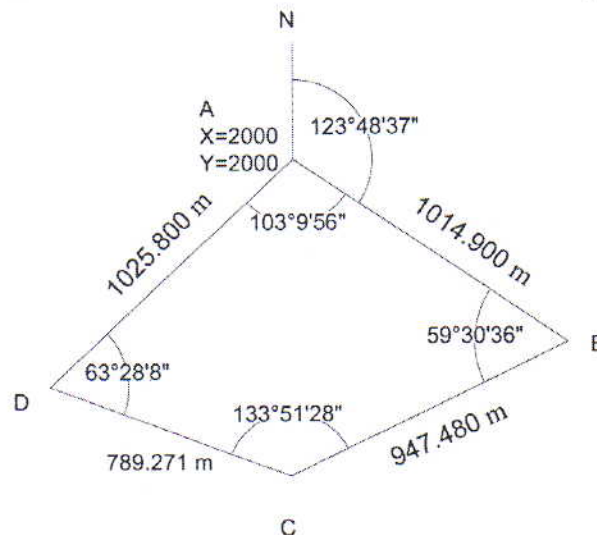
3 marks

13.771 m²
 11.94 m²

3. Compute the depth of cut and/or fill for all stations

Depth
2.5
1.641
2.273
1.771
1.586
2.267
0.873
2.064
2.161
1.334

Q2) For the following figure balance the departures and latitudes using the compass rule and computed the balanced coordinates of all points.



25 marks

$$\sum \text{The theoretical sum of interior angles} = (n - 2)180^\circ = (4 - 2)180^\circ = 360^\circ$$

$$T.C \text{ for interior angles} = \sum \text{Theor.} - \sum \text{Meas.} = 360^\circ - 360^\circ 00' 08'' = -8''$$

$$\text{Correction for single angle} = \frac{T.C. \text{ for Interior angles}}{n} = \frac{-8}{4} = -2''$$

P oint	Interior angle	Correct ion	Corrected interior angle
A	103° 09' 56"	-2'	103° 09' 54"
B	59° 30' 36"	-2'	59° 30' 34"
C	133° 51' 28"	-2'	133° 51' 26"
D	63° 28' 08"	-2'	63° 28' 06"

$$\sum \text{Meas.} = 360^\circ 00' 08'' \quad \sum -20' \quad \sum 360^\circ 00' 00''$$

Computations of Azimuths
Sides

Azimuth

AB	123° 48' 37"
BC	244° 18' 03"
CD	290° 26' 37"
DA	46° 58' 31"

point	Line	Az.	Length (m)	ΔX (m)	ΔY (m)	Corr ΔX	Corr ΔY
A							
	AB	123° 48' 37"	1014.9	843.264	-564.735	0.035	0.002
B							
	BC	244° 18' 03"	947.48	-853.758	-410.870	0.033	0.002
C							
	CD	290° 26' 37"	789.271	-739.559	275.680	0.027	0.002
D							
	DA	46° 58' 31"	1025.80	749.920	699.917	0.036	0.002
A							
			3777.451	-0.133	-0.008		

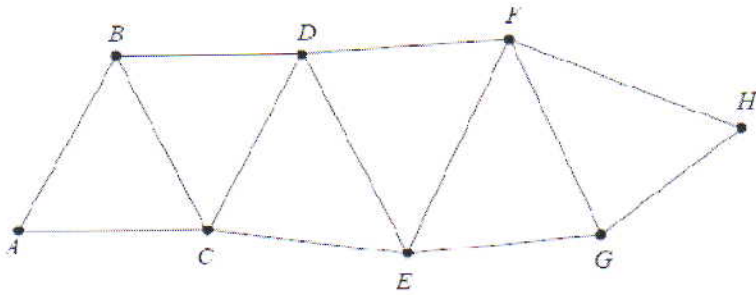
point	Line	Bal. ΔX (m)	Bal. ΔY (m)	X	Y
A				2000	2000
	AB	843.299	-564.733		
B				2843.299	1435.267
	BC	-853.725	-410.868		
C				1989.574	1024.399
	CD	-739.532	275.682		
D				1250.042	1300.081
	DA	749.956	699.919		
A				1999.998	2000

Q3) Answer two of the following: 12.5 marks each

A) In triangulation systems what are the types of layouts, explain with figures.

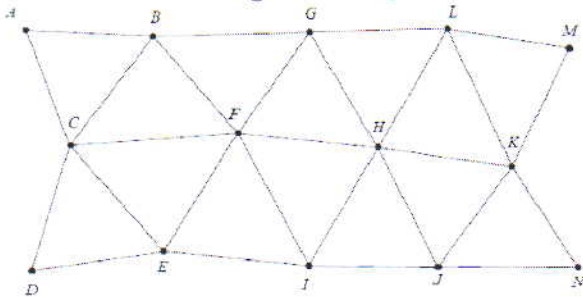
Single chain of triangles

When the control points are required to be established in a narrow strip of terrain such as a valley between ridges, a layout consisting of single chain of triangles is generally used as shown in following figure.



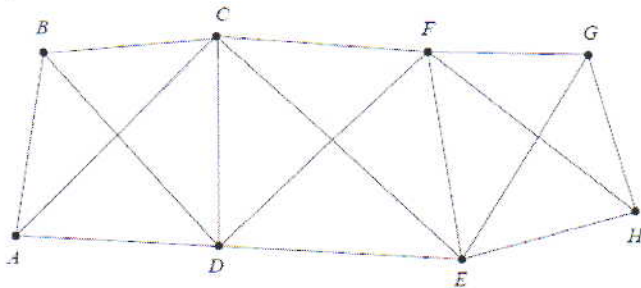
Double chain of triangles

A layout of double chain of triangles is shown in following Fig. This arrangement is used for covering the larger width of a belt.



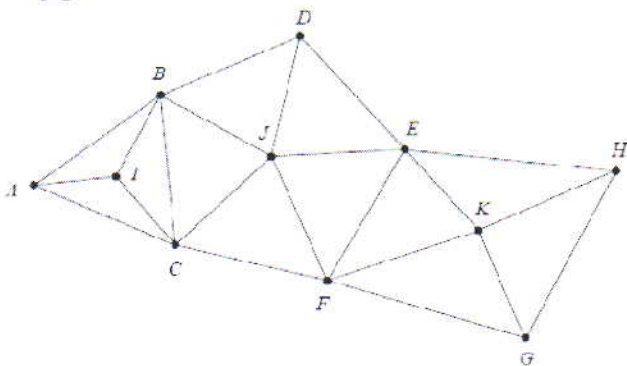
Braced quadrilaterals

A triangulation system consisting of figures containing four corner stations and observed diagonals shown in following Fig., is known as a layout of braced quadrilaterals.



Centered triangles and polygons

A triangulation system which consists of figures containing interior stations in triangle and polygon as shown in following Fig., is known as centered triangles and polygons.



A combination of all above systems

Sometimes a combination of above systems may be used which may be according to the shape of the area and the accuracy requirements.

B) Reciprocal leveling was conducted across a wide valley to determine the difference in level of points *A* and *B*, *A* situated on one side of the valley and *B* situated on the other. The following results on the staff held vertically at *A* and *B* from level stations 1 and 2, respectively, were obtained. The level station 1 was near to *A* and station 2 was near to *B*. If the elevation of *B* is 400.487 m above the MSL, what is the elevation of point *A*?

Instrument at	Staff reading on	
	A	B
1	1.641	2.193
2	2.141	2.701

$$\Delta h = \frac{(a_1 - b_1) + (a_2 - b_2)}{2}$$

$$a_1 = 1.641$$

$$a_2 = 2.141$$

$$b_1 = 2.193$$

$$b_2 = 2.701$$

$$\Delta h = -0.556 \text{ m}$$

$$\text{Elevation of } A = \text{Elevation of } B - \Delta h = 400.487 + 0.556 = 401.043 \text{ m}$$

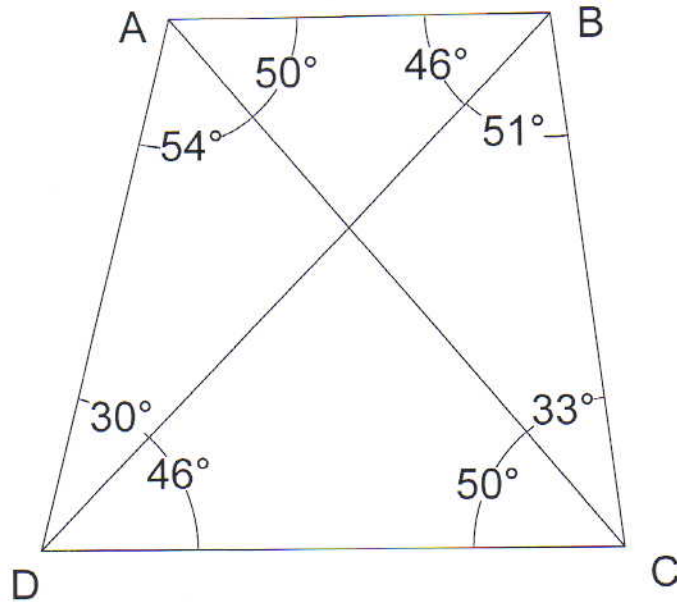
C) A steel tape standardized at 20°C, the tape had a weight of 0.03967 kg/m. This tape was held horizontally, supported at the ends only, with a constant tension of 9.09 kg, to measure a line from *A* to *B*, $D_{AB} = 50.205$. Apply corrections for tape sag, to determine the correct length of the line.

$$C_s = -\frac{w^2 L_s^3}{24 P_1^2}$$

$$C_s = -\left[\frac{(0.03967)^2 (50.205)^3}{24 (9.09)^2} \right] = -0.100 \text{ m}$$

Q4) For the following figure ABCD:

A) Compute the strength of the figure *ABCD* for all the routes by which the length *CD* can be computed from the known side *AB*. Assume that all the stations were occupied. 15 marks



$$R = \frac{D - C}{D} = \sum (\delta_A^2 + \delta_A \delta_B + \delta_A^2)$$

$$n = 6$$

$$n' = 6$$

$$S = 4$$

$$S' = 4$$

$$D = 2 \times (n - 1) = 2 \times (6 - 1) = 10$$

$$C = (n' - S' + 1) + (n - 2S + 3) = (6 - 4 + 1) + (6 - 2 \times 4 + 3) = 4$$

$$\frac{D - C}{D} = \frac{10 - 4}{10} = 0.60$$

(a) Route-1, using $\Delta^s ADC$ and ABC with common side AC

For ΔADC the distance angles of AD and AC are 50° and $76^\circ = 30^\circ + 46^\circ$, respectively.

From Table 1.2,

$$\delta_{50}^2 + \delta_{50} \delta_{76} + \delta_{76}^2 = 4$$

For ΔABC , the distance angles of AC and BC are $97^\circ = 51^\circ + 46^\circ$ and 50°

$$\delta_{97}^2 + \delta_{97} \delta_{50} + \delta_{50}^2 = 3$$

$$R = 0.6 * (4 + 3) = 4.2 \cong 4$$

(b) Route-2, using $\Delta^s ADC$ and DCB with common side DC

For ΔADC the distance angles of AD and DC are 50° and 54° , respectively.

$$\delta_{50}^2 + \delta_{50} \delta_{54} + \delta_{54}^2 = 8$$

For ΔDCB , the distance angle of DC and CB are 51° and 46°

$$\delta_{51}^2 + \delta_{51} \delta_{46} + \delta_{46}^2 = 11$$

$$R = 0.6 * (8 + 11) = 11.4 \cong 11$$

(c) Route-3, using $\Delta^s ADB$ and ACB with common side AB

From ΔADB the distance angles of AD and AB are 46° and 30° , respectively.

$$\delta_{46}^2 + \delta_{46} \delta_{30} + \delta_{30}^2 = 25$$

From ΔACB , the distance angles of AB and CB are 33° and 50° , respectively

$$\delta_{33}^2 + \delta_{50} \delta_{33} + \delta_{50}^2 = 20$$

$$R = 0.6 * (25 + 20) = 27$$

(d) Route-4, using $\Delta^s ADB$ and DCB with common side DB .

From ΔADB , the distance angles of AD and DB are 46° and $104^\circ = 50^\circ + 54^\circ$, respectively

$$\delta_{46}^2 + \delta_{46}\delta_{104} + \delta_{104}^2 = 4$$

From ΔDCB , the distance angles of DB and CB are $83^\circ = 33^\circ + 50^\circ$ and 46°

$$\delta_{83}^2 + \delta_{83}\delta_{46} + \delta_{46}^2 = 5$$

$$R = 0.6 * (4 + 5) = 5.4 \cong 5$$

B) If the horizontal coordinates of point $A=(X_A=10000 \text{ m}, Y_A=10000 \text{ m})$, length of line $AB = D_{AB}=5000 \text{ m}$ and Az of $AB= Az_{AB}= 190^\circ$. Compute the horizontal coordinates of the triangulation network using Δ^s **having the lowest value of R** .

10 marks

The lowest value of R is 4 for route 1

$\Delta^s ADC$ and ABC with common side AC

For ΔADC the distance angles of AD and AC are 50° and $76^\circ = 30^\circ + 46^\circ$, respectively.

$$D_{AD} = \frac{D_{AB} * \sin 46}{\sin 30} = 7193.398 \text{ m}$$

$$D_{AC} = \frac{D_{AD} * \sin 76}{\sin 46} = 9702.957 \text{ m}$$

For ΔABC , the distance angles of AC and BC are $97^\circ = 51^\circ + 46^\circ$ and 50°

$$D_{BC} = \frac{D_{AC} * \sin 50}{\sin 76} = 7660.444 \text{ m}$$

$$Az_{AD} = 294^\circ$$

$$Az_{AC} = 240^\circ$$

$$X_D = X_A + \Delta X = 3428.504 \text{ m}$$

$$Y_D = Y_A + \Delta Y = 12925.818 \text{ m}$$

$$X_C = X_A + \Delta X = 1596.997 \text{ m}$$

$$Y_C = Y_A + \Delta Y = 5148.521 \text{ m}$$

$$X_B = X_A + \Delta X = 9131.759 \text{ m}$$

$$Y_B = Y_A + \Delta Y = 5075.961 \text{ m}$$

Q5) A) List the typical total station programs

16 marks

Point Location

Trigonometric Leveling

Missing Line Measurement

Resection

Azimuth

Remote Object Elevation

Distance Offset Measurements

Angle Offset Measurements

Layout or Setting-Out Positions

9 marks

Area Computation

B) Using a programmed total station:

1- How would the surveyor can determine the heights of inaccessible points.

The surveyor can determine the heights of inaccessible points (e.g., electrical conductors, bridge components) by simply sighting the pole-mounted prism while it is held directly under the object. When the object itself is then sighted, the object height can be promptly displayed (the prism height must first be entered into the total station).

Known:

- N, E, and Z coordinates of the instrument station (\bar{A}).

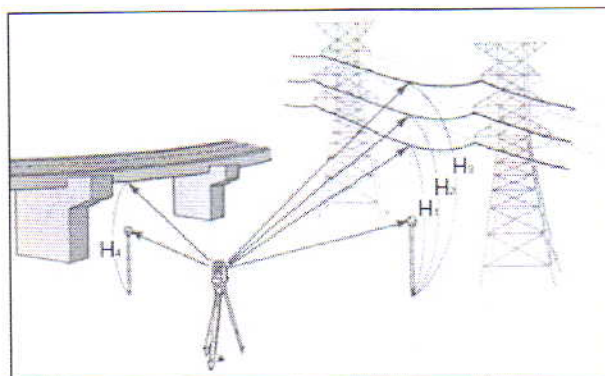
- N, E, and Z coordinates of a reference control point, or at least the azimuth of the line joining the instrument station and the control point.

Measured:

- Horizontal angle, or azimuth, from the reference control point (optional) and distance from the instrument station to the prism held directly below (or above) the target point.
- Vertical angles to the prism and to the target point h_i and height of the prism.

Computed:

- Distance from the ground to the target point (and its coordinates if required).



2- How would you find the center location of a concrete column.

When the center of a solid object (e.g., a concrete column, tree) is to be located, its position can be ascertained by turning angles from each side to the centerpoint. The software then computes the center location of the measured solid object.

Known:

- N, E, and Z coordinates of the instrument station (\bar{A}).
- N, E, and Z coordinates (or the azimuth) of the reference control point.

Measured:

- Angles from the prism being held on either side of the measuring point, to the target centerpoint. (The prism must be held such that both readings are the same distance from the instrument station—if both sides are measured).

Computed:

- N, E, and Z coordinates of the hidden measuring points.
- Azimuths and distance from the instrument station to the hidden measuring point.
- When the measuring point is to the center of a tree, some programs will use this application to determine tree's diameter and then to estimate the tree's height (knowing the height allows the CAD software to insert the correctly sized tree symbol on the digital drawing).

