



**University of Technology**  
**Building and Construction Eng. Dept.**  
**FINAL Exam –2015/2016**



**Subject : Theory of Structures**  
**Branch : Highway & Bridges Eng.**  
**Examiner : Dr. Qays Abdul-Majeed**

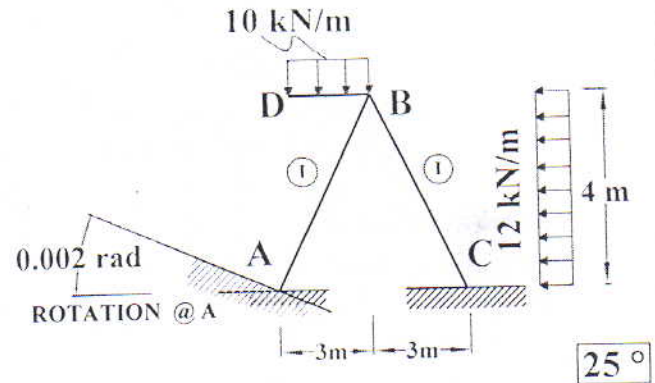
**Class : 3<sup>rd</sup> year**  
**Time : 3.0 Hours**  
**Date : 29 /05/2016**

**ANSWER 4 QUESTIONS ONLY**

**Q 1 :-**For the Frame shown :

- a) Discuss the stability and determinacy.
- b) Use Slope-Deflection Method to find end moments.
- c) Draw the bending moment diagram for member AB .

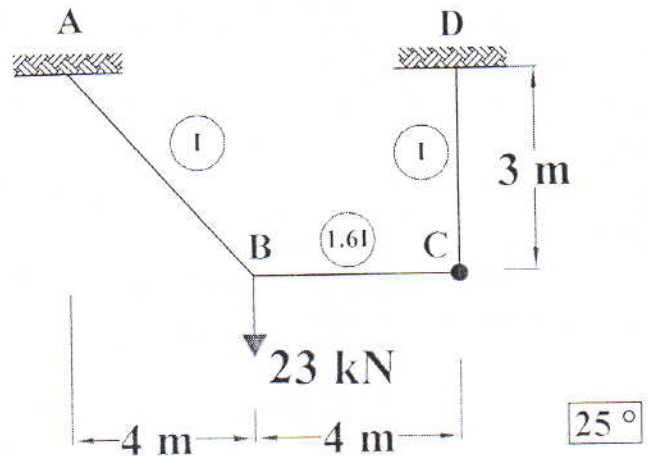
known that  $EI = 10^4 \text{ kN.m}^2$



**Q 2 :-**For the structure shown :

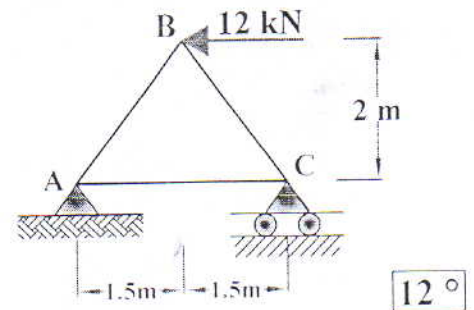
- a) Check the stability and determinacy.
- b) By using Moment Distribution Method find end moments.
- c) find Axial force in member CD.

known that  $EI = 10^4 \text{ kN.m}^2$

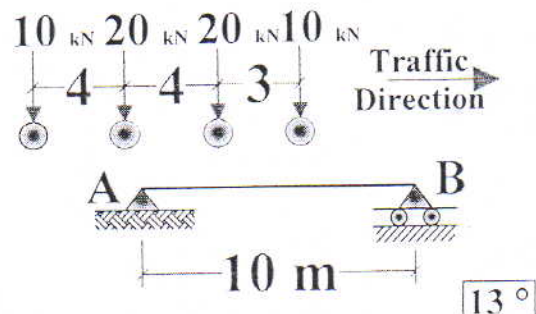


**Q3 :**

- a. Find the horizontal deflection at B " $\Delta_{Bh}$ " due to a horizontal force "12 kN" shown in the figure.  
 known that  $AE = 10^4 \text{ kN}$ .



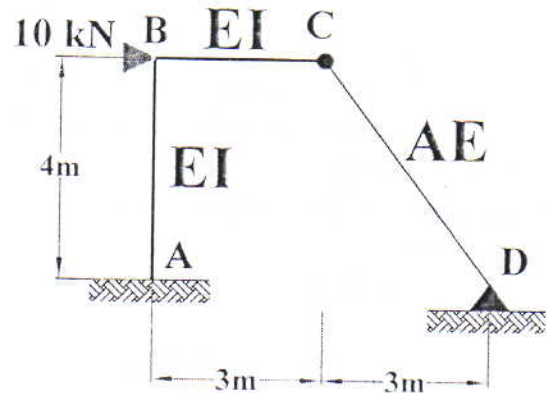
- b. Find absolute maximum moment for a simply supported beam of length of "10 m" due to moving load shown .



Q 4 :- For the structure shown :

- Check the stability and determinacy.
- By using Consistant Deformation Method find the force in link  $CD$ .
- Draw the Axial & Shear force Diagram for member  $BC$ .

known that  $EI = 10^4 \text{ kN.m}^2$  for  $AB$  &  $BC$   
 $AE = 10^4 \text{ kN}$  for  $CD$ .

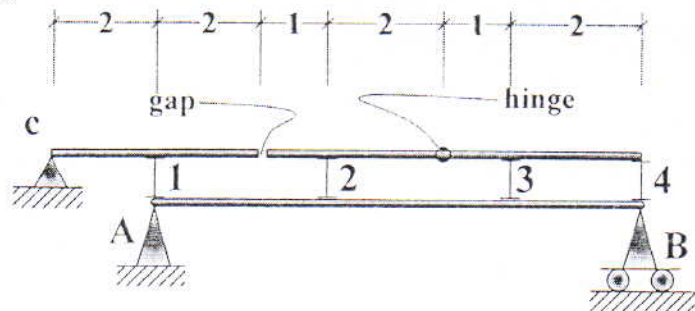
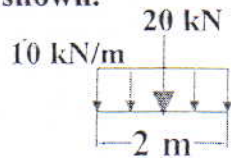


25°

Q 5 :-

a) For the structure shown :

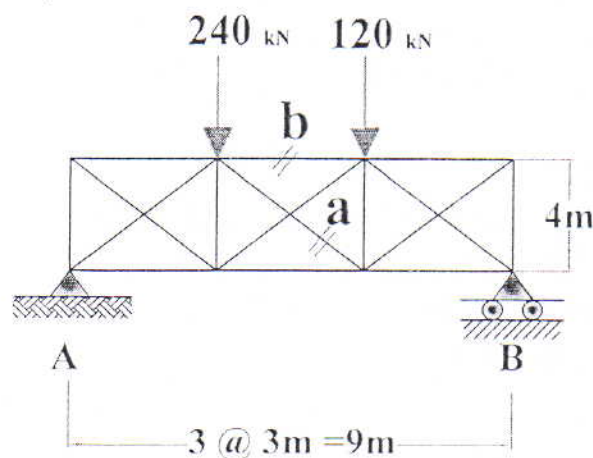
- Find maximum Reaction @ 3 due to moving load shown.



- Draw influence line for Shear between 2&3.

15°

- By using the Approximate Analysis find the forces in members ( a & b ).



10°

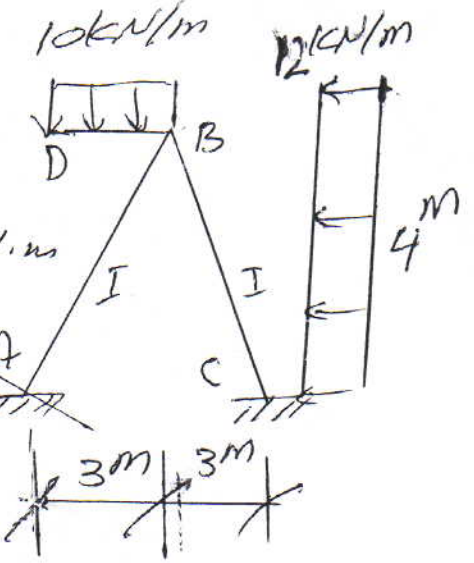
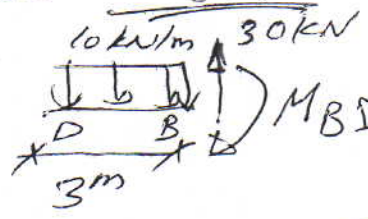
(1-8) حلول مسألة فرع الطرق واحسوا الزوايا الثلاثة

عادر الا تشادات الحد Q1

a) stable & Indet. to 3rd deg.

b)  $\sum M_B = 0$

$M_{BD} = 10(3)(1.5) = 45$   
kN.m



$\sum F_y = 0$

$V_B = 10(3) = 30$  kN

$M_{AB}{}_F = \frac{4(10^4)(0.002)}{5} = 16$  kN.m

$M_{BA}{}_F = \frac{2(10^4)(0.002)}{5} = 8$  kN.m

$M_{BC}{}_F = -\frac{12(4)^2}{12} = -16$  kN.m

$M_{CB}{}_F = \frac{12(4)^2}{12} = 16$  kN.m

انشه العنق  
لحوال بقا  
العودي  
لكن لان الاعمال اعقبة

$k_{rel}$	AB	BC
$\frac{I}{L}$	$\frac{1}{5}$	$\frac{1}{5}$
$K_{rel}$	1	1

معاداة التوازن  
حود للعض B

$M_{AB} = 16 + 1(0 + \theta_B) = 16 + \theta_B$   
 $M_{BA} = 8 + 1(2\theta_B + 0) = 8 + 2\theta_B$   
 $M_{BC} = -16 + (2\theta_B + 0) = -16 + 2\theta_B$   
 $M_{CB} = 16 + (0 + \theta_B) = 16 + \theta_B$

$\sum M_{joint B} = M_{ext} = 0$

$M_{BA} + M_{BC} + M_{BD} = 0 \Rightarrow 8 + 2\theta_B + (-16 + 2\theta_B) + 45 = 0$

$\therefore 4\theta_B = -37 \Rightarrow \theta_B = -9.25$  (relative rotation)

$\therefore M_{AB} = 6.75$  kN.m,  $M_{BA} = -10.5$  kN.m

$M_{BC} = -34.5$  kN.m,  $M_{CB} = 6.75$  kN.m

نتج (1-8)

C2-3.

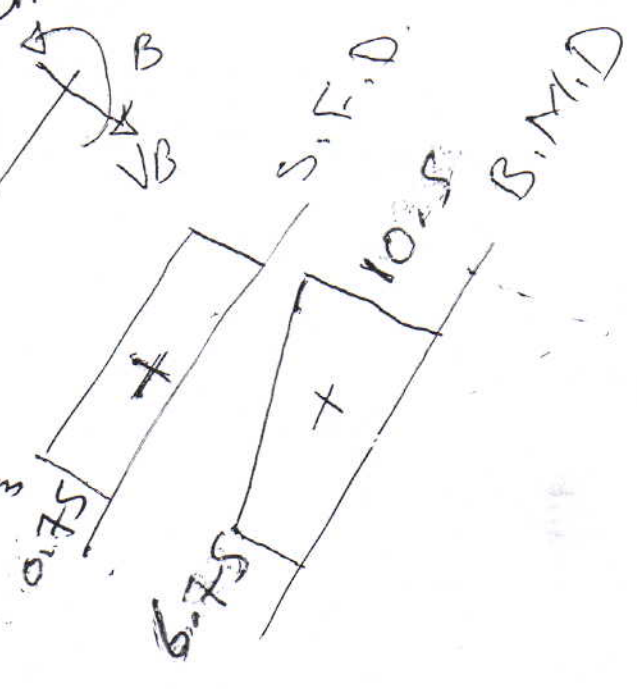
①  $V_A = V_B$  (سواء في المرفق أو في الطرف)  
 A B ج د هـ

$M_{BA} = 10.5 \text{ kN.m}$  (C.C.W)  $\frac{10.5}{5} = 2.1 \text{ kN}$   $\frac{10.5}{5} = 2.1 \text{ kN}$

②  $\sum M_B = 0$

$V_A = \frac{10.5 \times 5 - 6.75 \times 5}{5}$   
 $= 0.75 \text{ kN}$

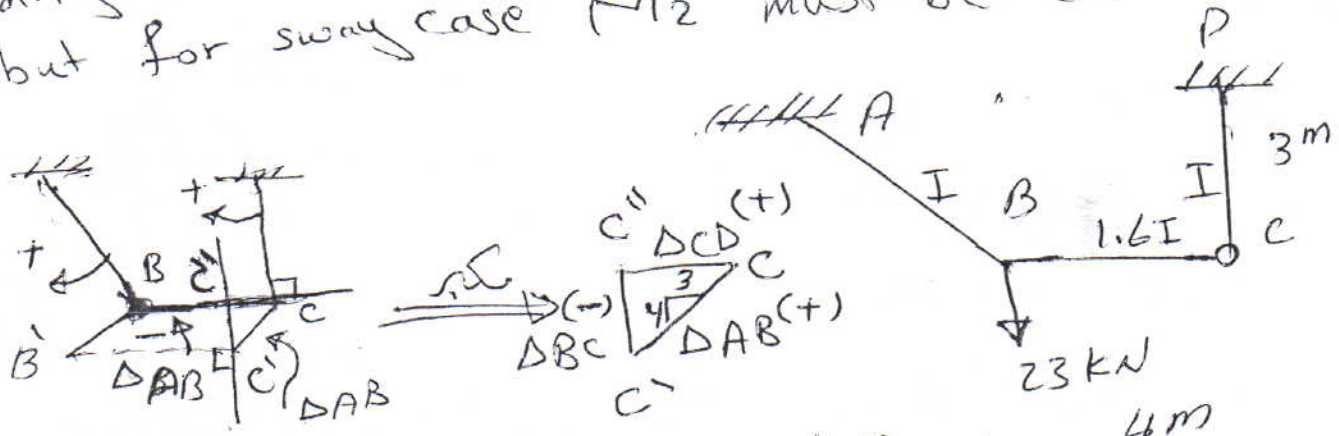
$M_{AB} = 6.75 \text{ kN.m}$  (C.W)



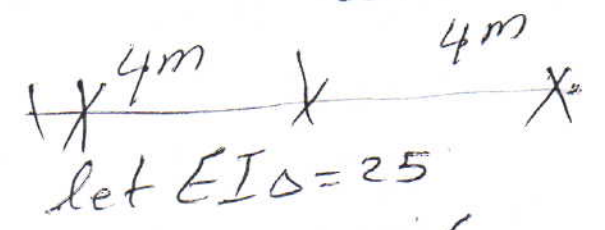
\* C.C.W  $\frac{10.5 \times 5}{2} = 131.25$   
 \* C.W  $\frac{6.75 \times 5}{2} = 168.75$

Q2 a) stable & Indet. to 2<sup>nd</sup> deg.

b) The structure has no external moment of loading cause moment. so  $M_1 = 0$   
 but for sway case  $M_2$  must be calculated



let  $\Delta_{AB} = +\Delta$  so  $\Delta_{BC} = -0.8\Delta$   
 $\Delta_{CD} = +0.6\Delta$



$M_{AB} = M_{BA} = -\frac{6EI\Delta}{5^2} = -\frac{6}{25}EI\Delta = -6$

$M_{BC} = M_{CB} = -\frac{6(1.6EI)(-0.8\Delta)}{4^2} = 0.48EI\Delta = 12$

$M_{CD} = M_{DC} = -\frac{6(EI)(0.6\Delta)}{2^2} = -0.4EI\Delta = -10$

حلولة المسئلة الثالثه من المرحله الثالثه مركز العرفه والحيوا  
(3-8)

ممكنه Q2 في الصيغه السابقه لحساب العزوم حسب الزوايا  
الاصليه ونح فرضاة  $EI \Delta = 25$  (أراي رقم امن) وكانت نتائج العزوم  
كما موضحة بالصيغة السابقه الان نحس حاصل التوزيع للمفرد B

$$k \cdot \frac{4(EI) \cdot 0.8EI}{5} = 0.8EI$$

$$k \cdot \frac{3(1.6EI)}{4} = 1.2EI$$

$$D.F. = \frac{k}{\Sigma k} = \frac{0.8EI}{2EI} = 0.4$$

$$= \frac{1.2EI}{2EI} = 0.6$$

توزيع التورج

Joint	A	B		C <sub>L</sub>	C <sub>R</sub>	D
Member	AB	BA	BC	CB	CD	DC
D.F.	0	0.4	0.6	1	1	0
F.E.M	-6	-6	12	12	-10	-10
D.M	0	-2.4	-3.6	-12	10	0
C.O.M	-1.2	0	-6		0	5
D.M	0	+2.4	+3.6		0	0
C.O.M	1.2	0	0		0	-
D.M	0	0	0		0	-
$\Sigma M_2$	-6	-6	6	0	0	-5
Maximal	-6k	-6k	6k			-5k

(3-8)