



University Of Technology
Building and Construction Eng. Dept.
Final Exam-First Attempt-2013/2014

Branch :Structural Eng.
subject : Sanitary Engineering
Examiner : Lec. Rana J.Kadhim

Class: Third
Time : 3 Hours
Date : 29/5/2014



Answer Four questions only

Q1:-A/ Compare the contact times necessary to give E. coli kills of 99.99 percent in water with free chlorine residual of 0.2 mg/l and combined chlorine residual of 1 mg/l , k values are 10^{-2} and 10^{-5} /s respectively.
(13marks)

B/ Explain in details two of the following:

- 1- The methods used for population forecastings.
- 2- The types and sources of water impurities.
- 3- The characteristics of sand and gravel used in filter media.

(12marks)

Q2:- A/ Determine the maximum daily consumption and fire demand in (l/c/d) for a community of 22000 capita, has an average Consumption is 600 l/c/d and fire flow dictated by a 6 story ordinary construction building of a floor area of 1000m²/story.

(13marks)

B/ Explain briefly of the following:

- 1- Biochemical oxygen demand (BOD).
- 2- The factors that affect process of self – purification of stream.
- 3- Breakpoint chlorination.

(12marks)

Q3:-A/ In a rectangular sed. Tank , inlet velocity 25 cm/ min , L:W 2:1 , effluent weir length 60 m, detention time 2 hr , the smallest particle to be removed 100% is 0.06 mm in diameter , S.G.= 1.2 , $\rho_w = 1 \text{ gm/ cm}^3$, $\mu = 1.03 * 10^{-2} \text{ gm/ cm.s.}$ find:

- 1- SOR (m/d).
- 2- Dimension of tank (m).
- 3- Weir loading (m³/m/d).

(13marks)

B/ Draw a sketch showing physical and biological processes (units) of conventional waste water treatment plant and describe the benefits of each unit.

(12 marks)

Q4:- A/ A stream with BOD 2 mg/l and saturated with DO has a normal flow of $2.26 \text{ m}^3/\text{s}$ and receives a sewage effluent, also saturated with DO, of $0.755 \text{ m}^3/\text{s}$ with BOD 30 mg/l. Determine the DO deficits over the next five days and hence plot the sag curve. Calculate the critical DO deficit and the time at which it occurs. Assume temperature is 20°C throughout. Saturation DO at 20°C is 9.17 mg/l, K_1 for effluent / water mixture is 0.17 / day, K_2 for stream is 0.4 / day.

(13marks)

B/ Write short notes on the following :-

- 1- The factors that affect the coagulation process.
- 2- The common methods used for disinfection of water.
- 3- The method of filtration and backwashing of filters and show it by drawings of rapid sand filter.

(12marks)

Q5:-A/ Rapid sand filter $L=9\text{m}$, $W=4.5\text{m}$ after filtering $20000 \text{ m}^3/\text{d}$ in 24 hr period, the filter is backwash at a rate $0.6 \text{ m} / \text{min}$. The filter is designed with 4 trough find :-

- 1- Filtration rate.
- 2- Quantity of wash water flow in each trough.
- 3- Dimension of trough of square cross section.

(13marks)

B/ Draw a sketch showing bacterial growth curves based on (number and mass of organisms).

(12marks)

Q1/A/ Sol. :

$$a) t^2 = \frac{2}{k} \log \frac{N_0}{N_t}$$

$$kill = 1 - \frac{N_t}{N_0}$$

$$0.9999 = 1 - \frac{0.2}{N_0}$$

$$N_0 = 2000 \text{ mg/l}$$

$$t^2 = \frac{2}{10^{-2}} \log \frac{2000}{0.2}$$

$$t = 28 \text{ s}$$

$$b) 0.9999 = 1 - \frac{1}{N_0}$$

$$\frac{1}{N_0} = 10^{-4} \Rightarrow N_0 = 10000$$

$$t^2 = \frac{2}{10^{-5}} \log \frac{10000}{1}$$

$$t = 894 \text{ s}$$

Q1/B/

① ① Graphical method, Arithmetic method

$$\frac{dp}{dt} = k_a$$

$$dp = k_a dt$$

$$P_f = P_i + k_a (t_f - t_i)$$

$$\therefore k_a \frac{P_f - P_i}{t_f - t_i} = \frac{P_i - P_e}{t_i - t_e}$$

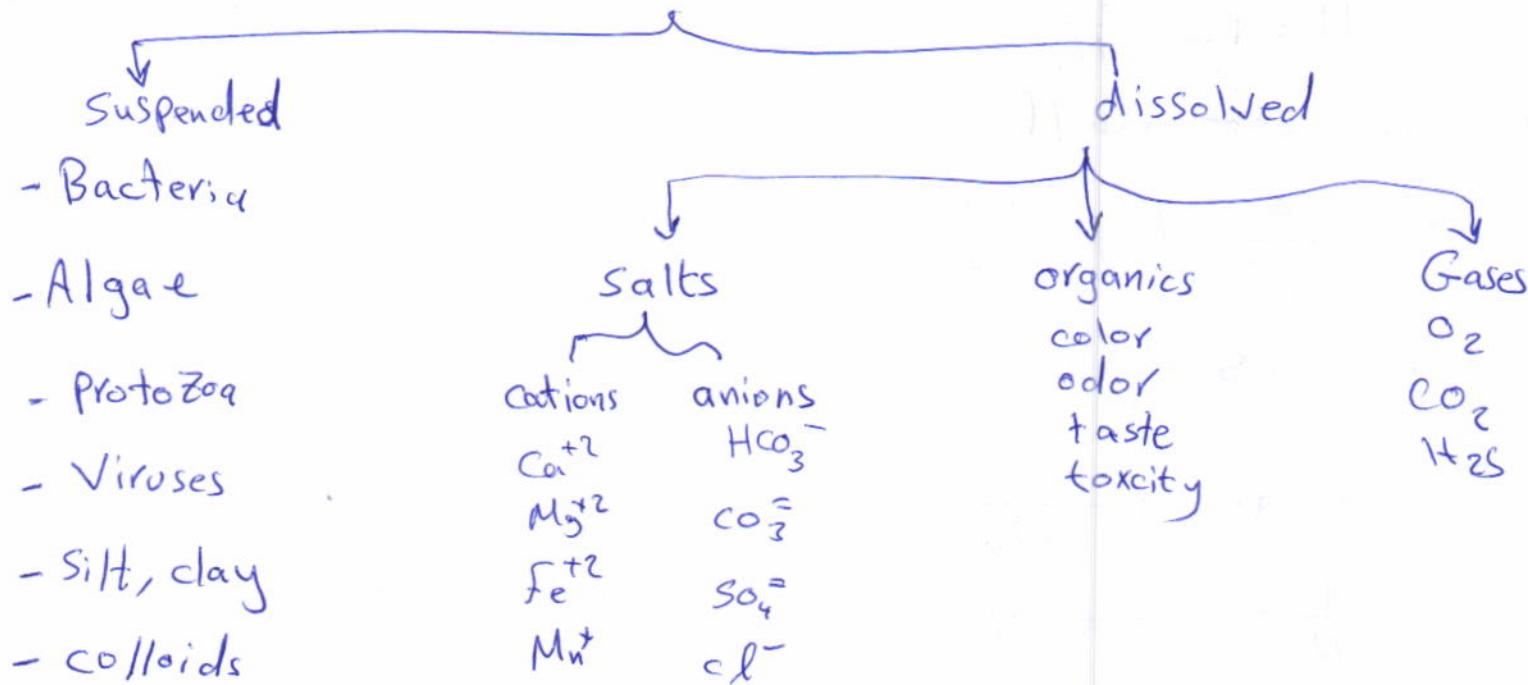
- Geometrical method: $\frac{dp}{dt} = k_g P$

Comparative method

Ratio and Correlation method:

$$\frac{P_j}{P_j'} = \frac{P_i}{P_i'} = k \text{ (constant)}$$

2/ Impurities of water



3/ صفات الرمل والحصى

الرمل: أ، لها مادة كوسط للتدريج، خالي من الأوساخ، هلب، مقاوم بلوري، لا يفقد أكثر من 5% من الوزن بعد غسله بحامض HCl نسبة 40% طرية في ساحة، بحف الرمل 60-70 سم ولحم مؤثر 0.45 - 0.55 mm حاصل التآكل لا يتجاوز $1.7 \leq U.C. \leq 1.2$

الحصى: يوضع الحصى تحت أوسعة طبقات حيث تكون الطبقات العليا حاوية على الحصى (هلب)، هلب، صلب، مقاوم، الوزن التقريبي 1600 كغم/م³ خالي من الشوائب، غير حاوي على الألياف، الرمال، تراب زجاجية، قواقع أو أي مواد غريبة، بحف 400 - 600 mm

Q2/A/ sol.

Average domestic demand = $22000 \times 600 = 13.2 \times 10^6 \text{ l/d}$

Maximum daily demand = $1.8 \times \text{ave.} = 23.76 \times 10^6 \text{ l/d}$

$$F = 180 \sqrt{A}$$

$$= 18(1) \sqrt{1000 \times 10.76 \times 6}$$

$$= 4574 \text{ gpm} = 17288 \text{ l/min} = 24.89 \times 10^6 \text{ l/d}$$

$$\text{Maximum Rate} = 23.76 \times 10^6 + 24.89 \times 10^6$$

$$= 48.65 \times 10^6 \text{ l/d}$$

$$= 2211 \text{ l/c/d For 10 hours}$$

The total flow required during this day would be: -

$$23.76 + 24.89 \times \frac{10}{24} = 34.13 \times 10^6 \text{ l}$$

$$= 1551 \text{ l/c/d}$$

Q2) B/

- 1- BOD: Bacteria placed in contact with organic material. will utilize it as a food source. In the utilization the organic material oxidized to stable end product such as CO_2 & water. The amount of oxygen used in this process is called BOD and is considered to be a measure of the organic content of the water.

2)

1- Dilution

2- Current assist in dispersion

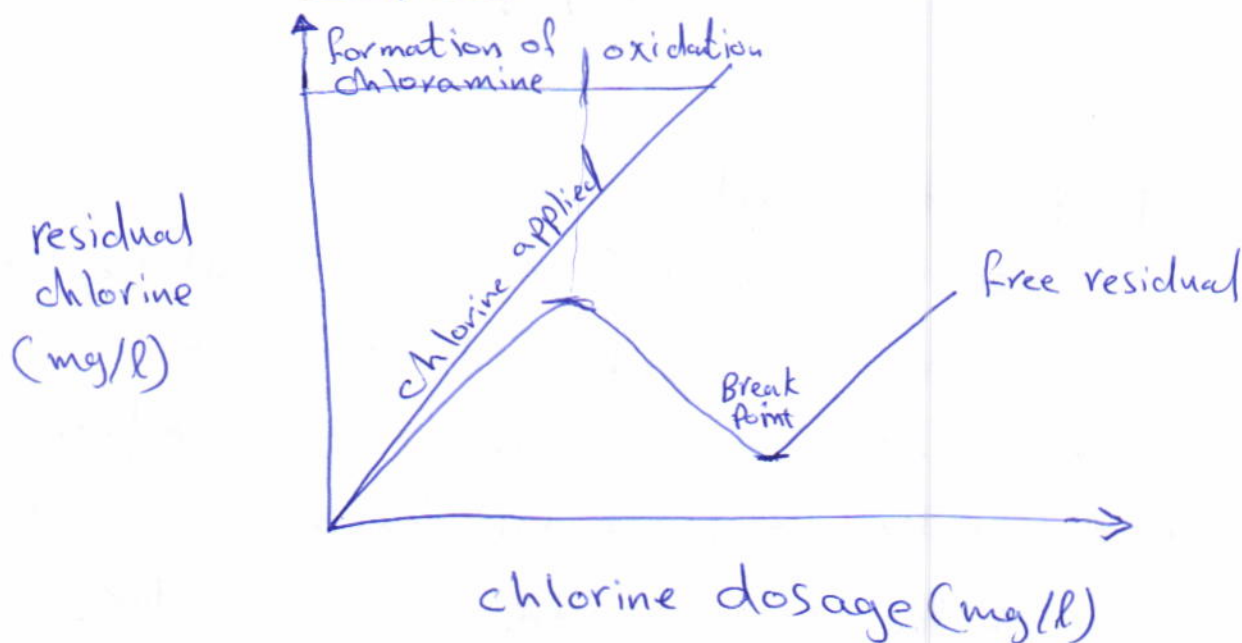
3- Sedimentation - Suspended Solids are removed by settling if the stream velocity is less than the scour velocity

4- Bottom deposits and runoff

5- Sunlight.

6- Temperature.

3) The chlorine demand of water depends upon the organic and inorganic substance present --- chloramine & other chloroorganics compounds, the oxidation complete rise in the chlorine residual, free chlorine, the point at which this take place is called "break point"



Q3) A/

$$\textcircled{1} V_s = \frac{g}{18\mu} (S.G. - 1) \rho_w \cdot d^2$$

$$= \frac{980}{18 \times 103} (100)(1.2 - 1)(1) \left(\frac{0.06}{10} \right)^2$$

$$V_s = 0.038 \text{ cm/s} = 0.38 \times 10^{-3} \text{ m/s} = 1.368 \text{ m/hr} \\ = 32.8 \text{ m/d} = \text{SOR}$$

$$\textcircled{2} V_h = 0.25 \text{ m/min} = 0.25 \times 60 = 15 \text{ m/hr}$$

$$V_h = \frac{Q}{WH} \Rightarrow Q = 15 WH \text{ m}^3/\text{hr}$$

$$\text{SOR} = \frac{Q}{WL} \Rightarrow Q = 1.368 WL \text{ m}^3/\text{hr}$$

$$Q = \frac{\text{Vol.}}{\text{time}} = \frac{WLH}{2} \quad L = 2W$$

$$\therefore Q = \frac{2W^2H}{2} = W^2H \text{ m}^3/\text{hr}$$

$$15 WH = W^2H \Rightarrow \begin{array}{|l|} \hline W = 15 \text{ m} \\ \hline L = 30 \text{ m} \\ \hline \end{array}$$

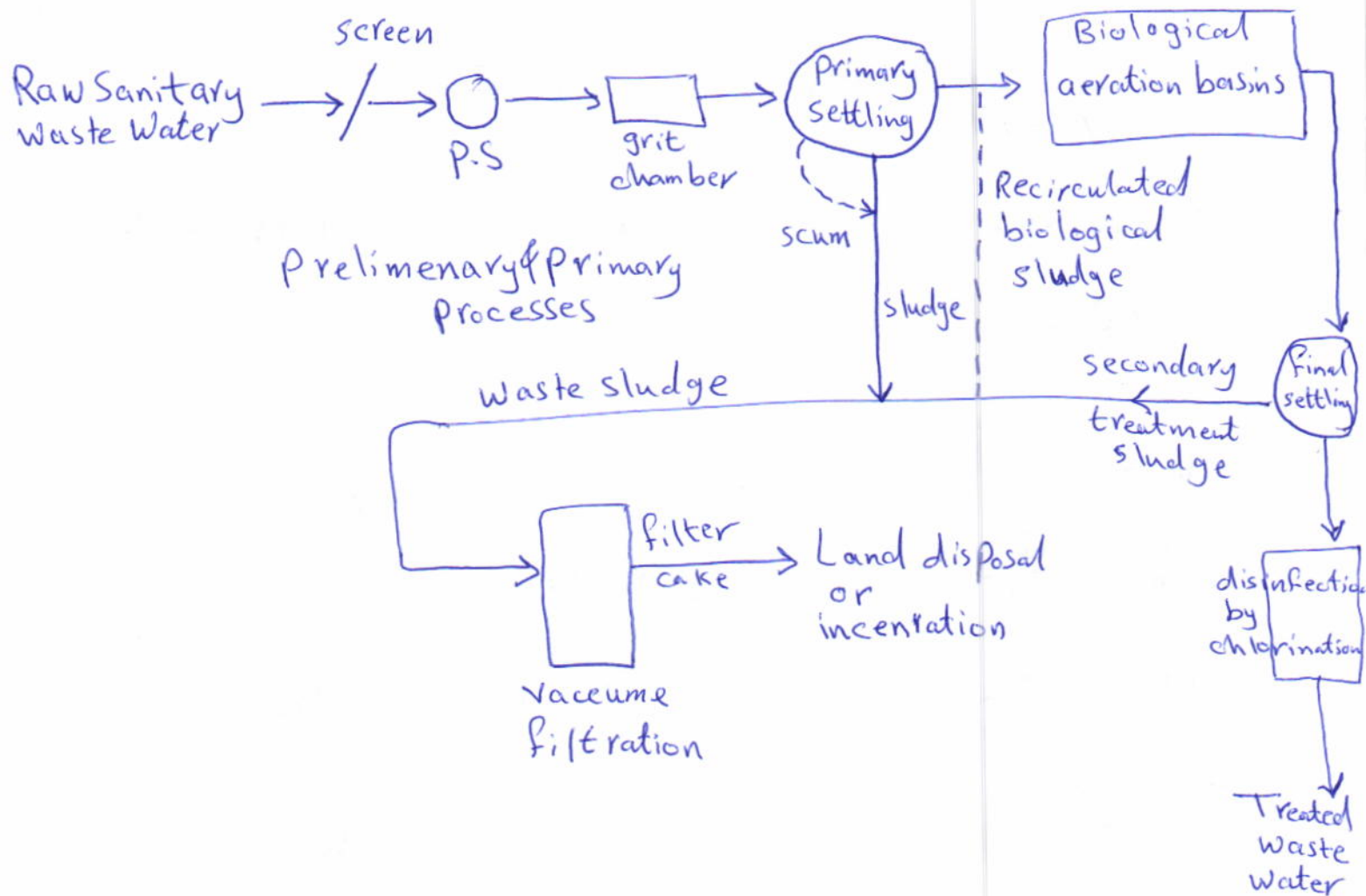
$$Q = W^2H \Rightarrow 615.6 = (15)^2 H \Rightarrow \boxed{H = 2.74 \text{ m}}$$

$$\textcircled{3} \text{ Weir loading} = \frac{Q}{\text{Weir length}}$$

$$= \frac{615.6 (24)}{60}$$

$$= 246.24 \text{ m}^3/\text{m/d}$$

Q3 B/



Q4) A/ Sol.

$$BOD_{min} = \frac{2.26(2) + 0.755(30)}{2.26 + 0.755} = 9 \text{ mg/l}$$

$$L_a = BOD_{ult} = \frac{BOD_{mix}}{1 - 10^{-k_1 t}} = \frac{9}{1 - 10^{-0.17 \times 5}}$$

$$= 10.48 \text{ mg/l}$$

كلية، لعلاقة التآكلية بـ الجذر خلال 5 أيام

$$D_t = \frac{k_1 L_a}{k_2 - k_1} (10^{-k_1 t} - 10^{-k_2 t}) + D_a 10^{-k_2 t}$$

$$D_a = D_s - P_r = 0$$

$$t_c = \frac{1}{k_2 - k_1} \log \left\{ \frac{k_2}{k_1} \left[1 - \frac{D_u(k_2 - k_1)}{L_a k_1} \right] \right\}$$

$$t_c = \frac{1}{0.4 - 0.17} \log \left\{ \frac{0.4}{0.17} \right\} = 1.61 \text{ day}$$

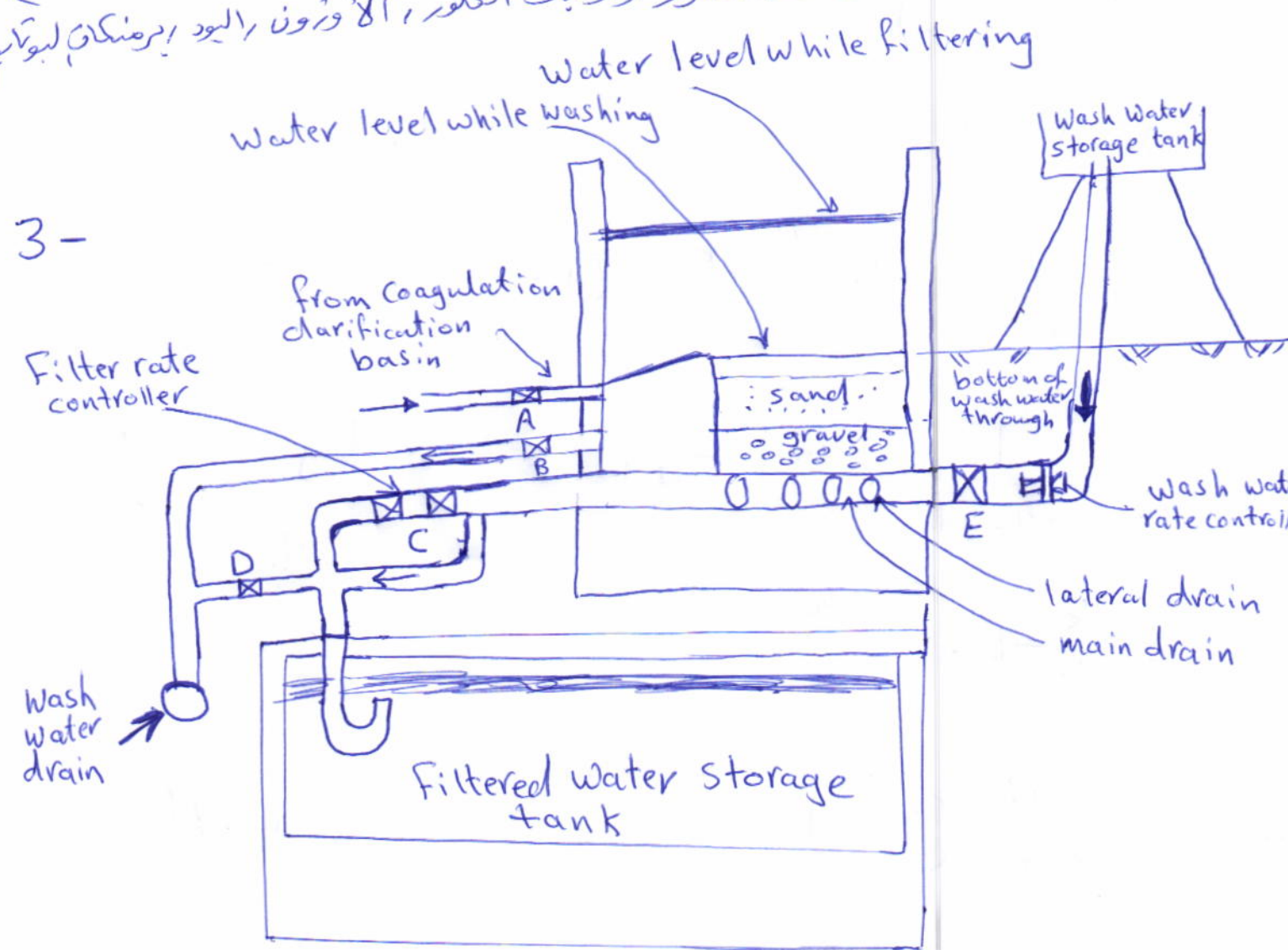
$$D_c = \frac{k_1}{k_2} L_a 10^{-k_1 t_c}$$

$$D_c = \frac{0.17}{0.4} (10.48) (10)^{-0.17 \times 1.61} = 2.36 \text{ mg/l}$$

Q4) B/ 1-

توجيه الماء ، كمية وفواهم المواد الغروية
pH ، الخبث السريع وفرة الكليد وسرعة الجاذبية
درجة الحرارة ، القاعدية ، فواهم لا يونات في الماء

- 2 - وسائل التقييم : ① الطرق الفيزيائية وتتم المعايير بالكمية وترفع درجة الحرارة الى 100 م و لمدة 15 - 20 دقيقة لعقد الجرائم
- استبعاد الأشعة فوق البنفسجية حيث تتركز المياه غير خروقة لغيرها في الأشعة فوق البنفسجية بطول موجة 200 - 360 نانومتر .
 - استبعاد ايونات المعادن مثل الفضة والنحاس
 - أشعة سونار
- ② الطرق الكيميائية : وتتم المواد الكيميائية المتوكدة (مركبات تستقبل الالكترونات) لتقدير المياه ، ومثال ذلك غاز الكلور ومركبات الكلور ، الأوزون ، اليود ، برمنجنات البوتاسيوم



Q5) A/ sol.

$$A_s = W \times L$$

$$= 4.5 \times 9$$

$$= 40.5 \text{ m}^2$$

① Filtration rate

$$V_f = \frac{Q}{A_s}$$

$$= \frac{20000}{40.5}$$

$$= 493.8 \text{ m/d}$$

② Quantity of washwater flow in each trough = $V_b \cdot A_s \cdot t \cdot \text{no. of filter}$

Assume $t = 15 \text{ min}$

$$= 40.5 \times 0.6 \times 15 \times 1$$

$$= 364.5 \text{ m}^3$$

③ $Q = \frac{V_b \cdot A_s}{\text{no. of trough}}$

$$= \frac{0.6 \times 40.5}{4}$$

$$= 6.075 \text{ m}^3/\text{min} = 0.101 \text{ m}^3/\text{s}$$

$$y = 1.73 \sqrt{Q^2 / 9y^2}$$

$$y^5 = (1.73)^3 (Q^2) \frac{1}{g} = (1.73)^3 (0.101)^2 \left(\frac{1}{9.81} \right)$$

$$y = b = 0.352 \text{ m} = 35.2 \text{ cm}$$

Q5 B/

